

Competitive comparison of load combination models



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Introduction

Comparison based on previous experience

Numerical example

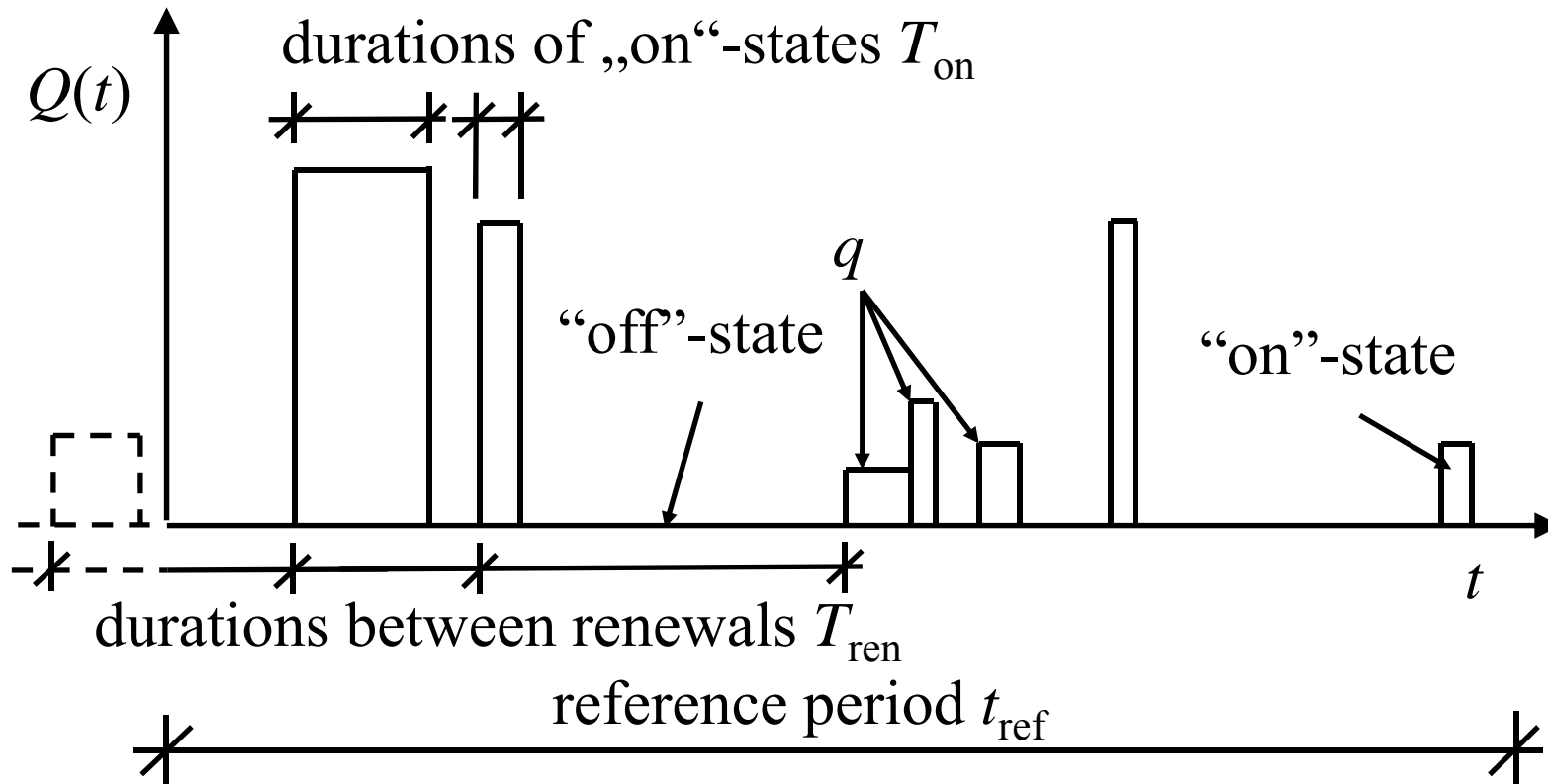
Concluding remarks

Introduction

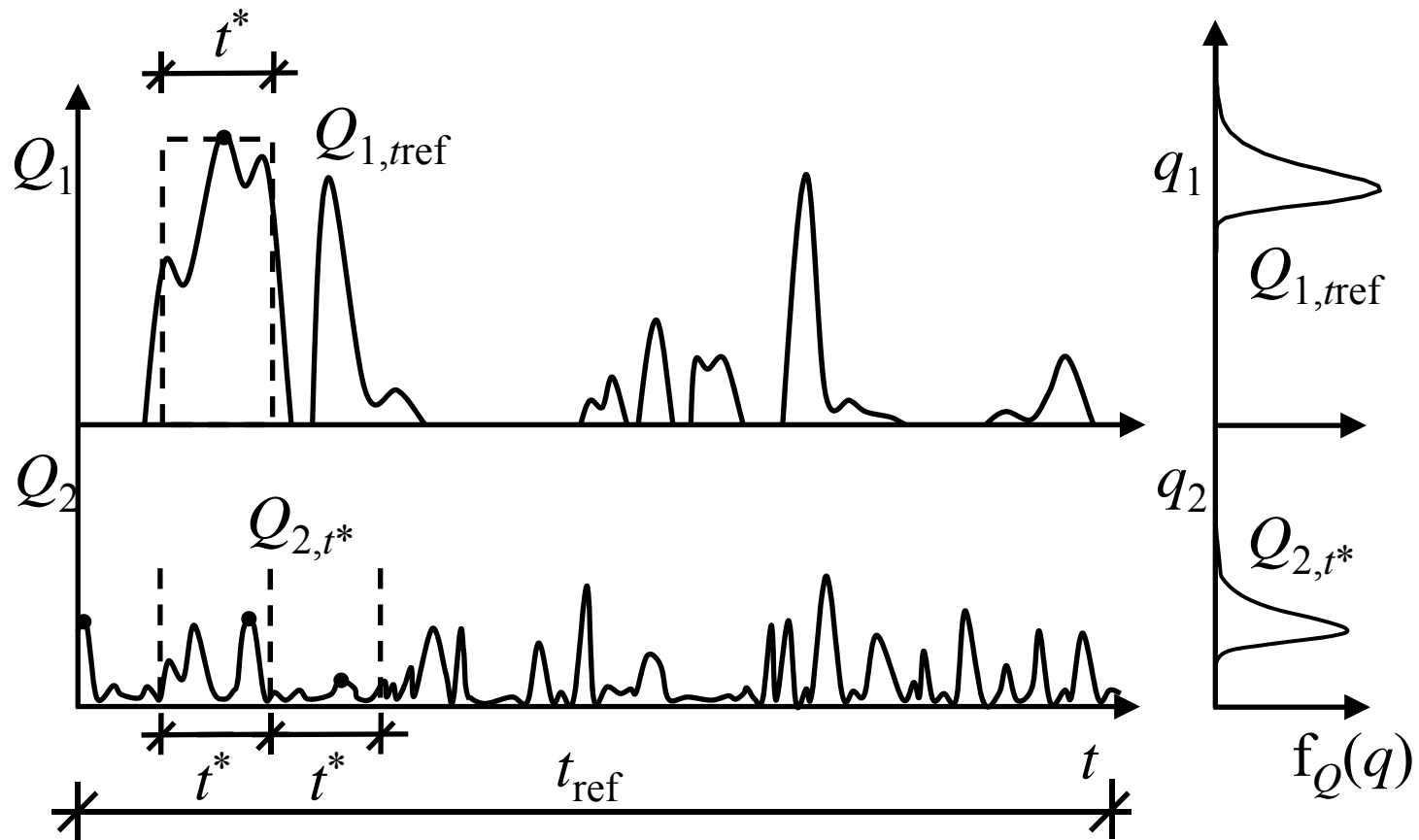
- Civil engineering structures often exposed to *combinations of time-variant loads* (climatic actions, imposed loads)
- *Several* load combination *models* applied in reliability studies
- The present study aimed at comparison of *three selected approaches*:
 - Rule proposed by *Turkstra* (1970)
 - Rectangular wave renewal processes with fixed durations of pulses, Ferry Borges & Castanheta (1971) – *FBC models*
 - Rectangular wave *renewal processes* with random durations between renewals and random durations of load pulses, Rackwitz (1998) and Sykora (2005)
- Comparison based on *previous experience, numerical study*

Basic assumptions

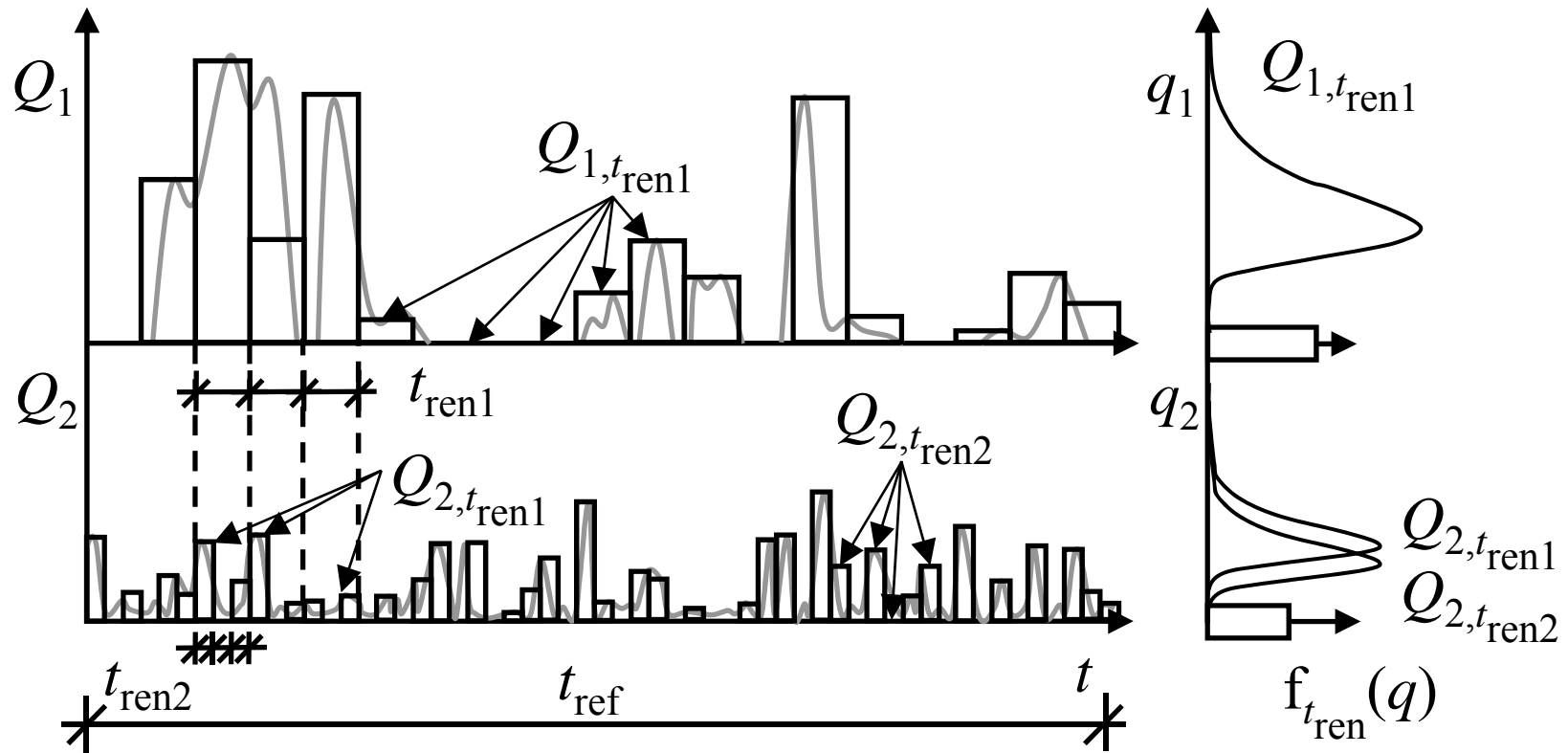
- *Resistance*, geometry variables, permanent actions and model uncertainties - *time-invariant*
- *Time-variant actions* described by stationary, ergodic and regular processes



Turkstra's rule

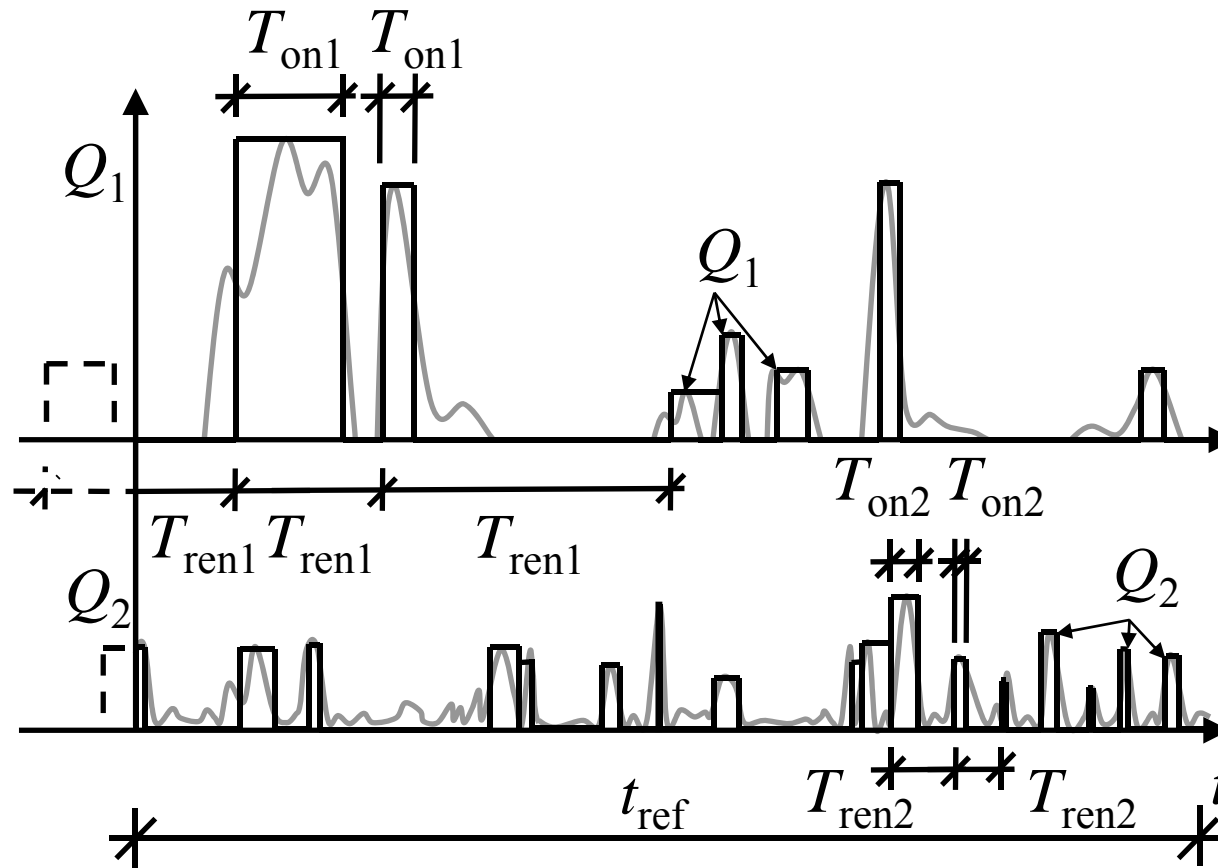


FBC models



Renewal processes

- *Upper bound* on the failure probability in most applications (initial failure probability + outcrossing rate)



Comparison based on previous experience

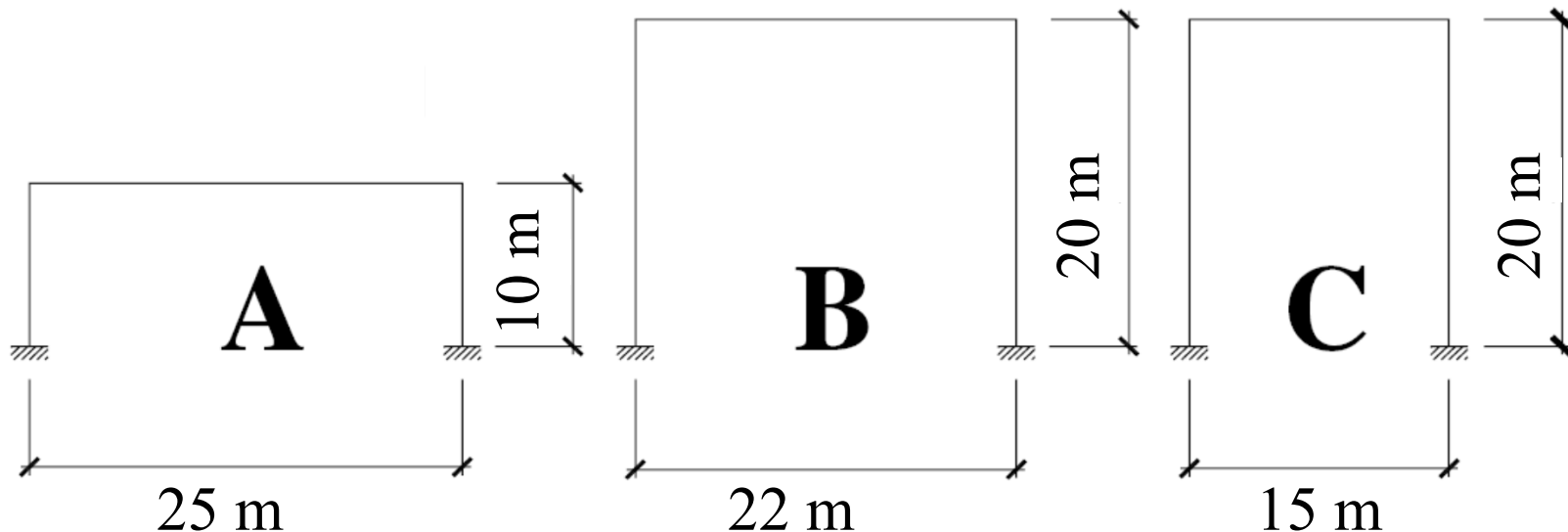
- Applicability of reliability methods
 - (+) *Turkstra* - any of well-established methods for the time-invariant analysis
 - (-) *FBC models* – Rackwitz-Fiessler algorithm available in few software products
 - (-) *Renewal processes* – upper bound unavailable in software products
- Accuracy
 - (0) *Turkstra* – sufficiently accurate in most cases (given the leading action is identified)
 - (0) *FBC models* – exact solution (applicability to short-term actions like storms and earthquakes disputable)
 - (0) *Renewal processes* – applicable for many types of actions, crude approximation when time-invariant variables dominant

Comparison based on previous experience

- Estimation of partial factors (calibration studies)
 - (+) *Turkstra* - straightforward
 - (-) *FBC models* – easy for time-invariant variables, difficulties for time-variant loads
 - (0) *Renewal processes* – straightforward when a dominant load case can be identified
- Non-stationary cases (out of the scope of the contribution)
 - (-) *Turkstra* and *FBC models* – upper bound (maximum load effect and minimum resistance) may be overly conservative
 - (+) *Renewal processes* – efficient analysis using the Laplace transform

Numerical example

- Reliability analysis of low-rise frames exposed to *snow* and *wind*, Schleich et al. (2002) and Sadovsky & Pales (2008)
- Design according to Eurocodes
- Models for the *monthly maxima* of the climatic loads - meteorological data for six locations in Germany
- *Snow* present with the *probability* p_{on} ; wind always present



Basis of analysis

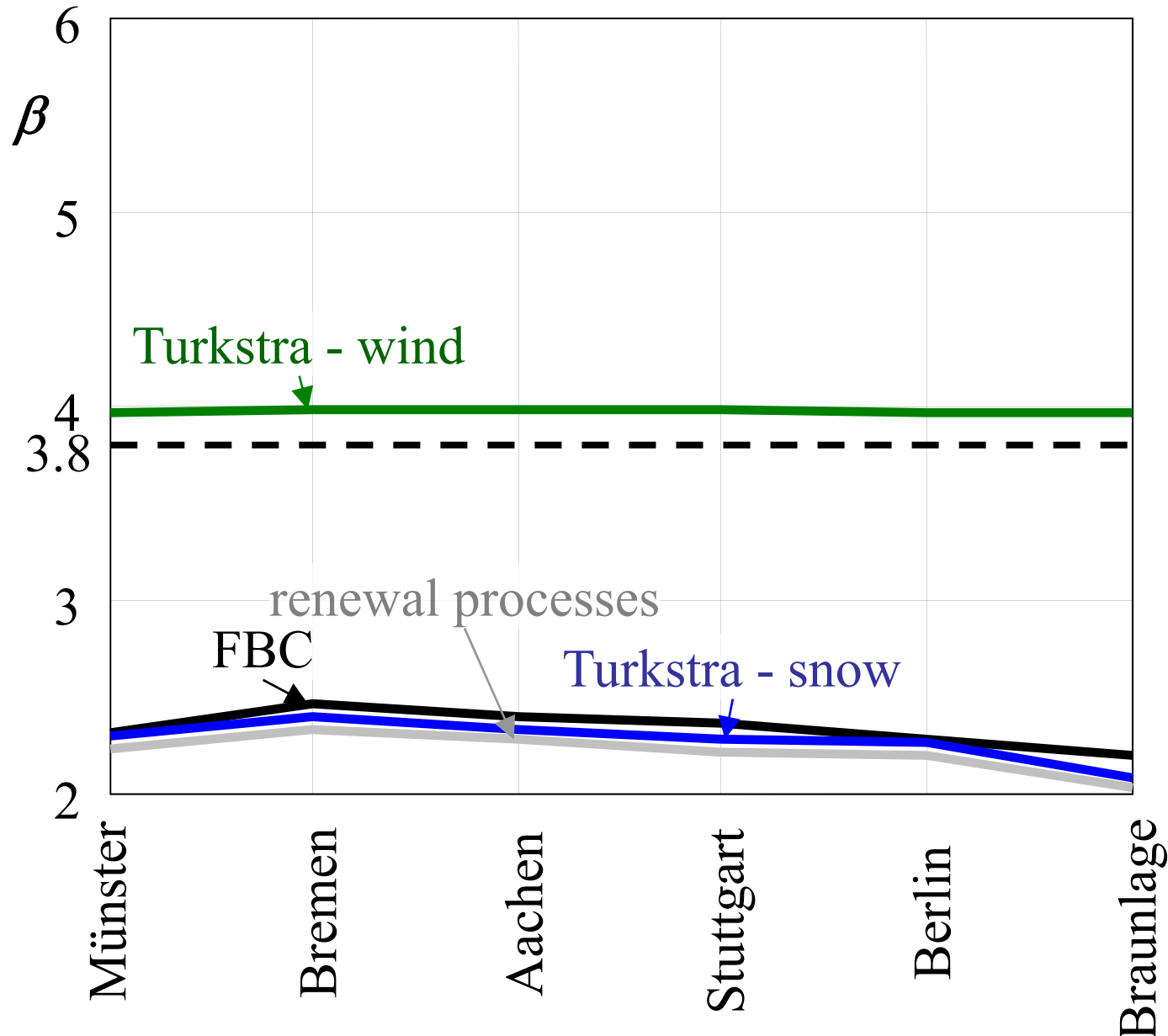
- *Limit state function*: $g[\mathbf{X}(t)] = K_R R - K_E [G + S(t) + W(t)]$
- Reference period – 50 years

Variable	Dist.	μ_X/x_k	V_X	$p_{on,X}$
Resistance R	LN	1.18	0.08	-
Permanent load G	N	1	0.10	-
Snow on roof S (Münster)	GU	0.26	1.17	0.23
Wind action W (Münster)	GU	0.17	0.67	1
Resistance uncertainty K_R	LN	1.15	0.05	-
Load effect uncertainty K_E	LN	1.0	0.10	-

- Parameter - *load ratio* $\chi = (s_k + w_k) / (g_k + s_k + w_k)$

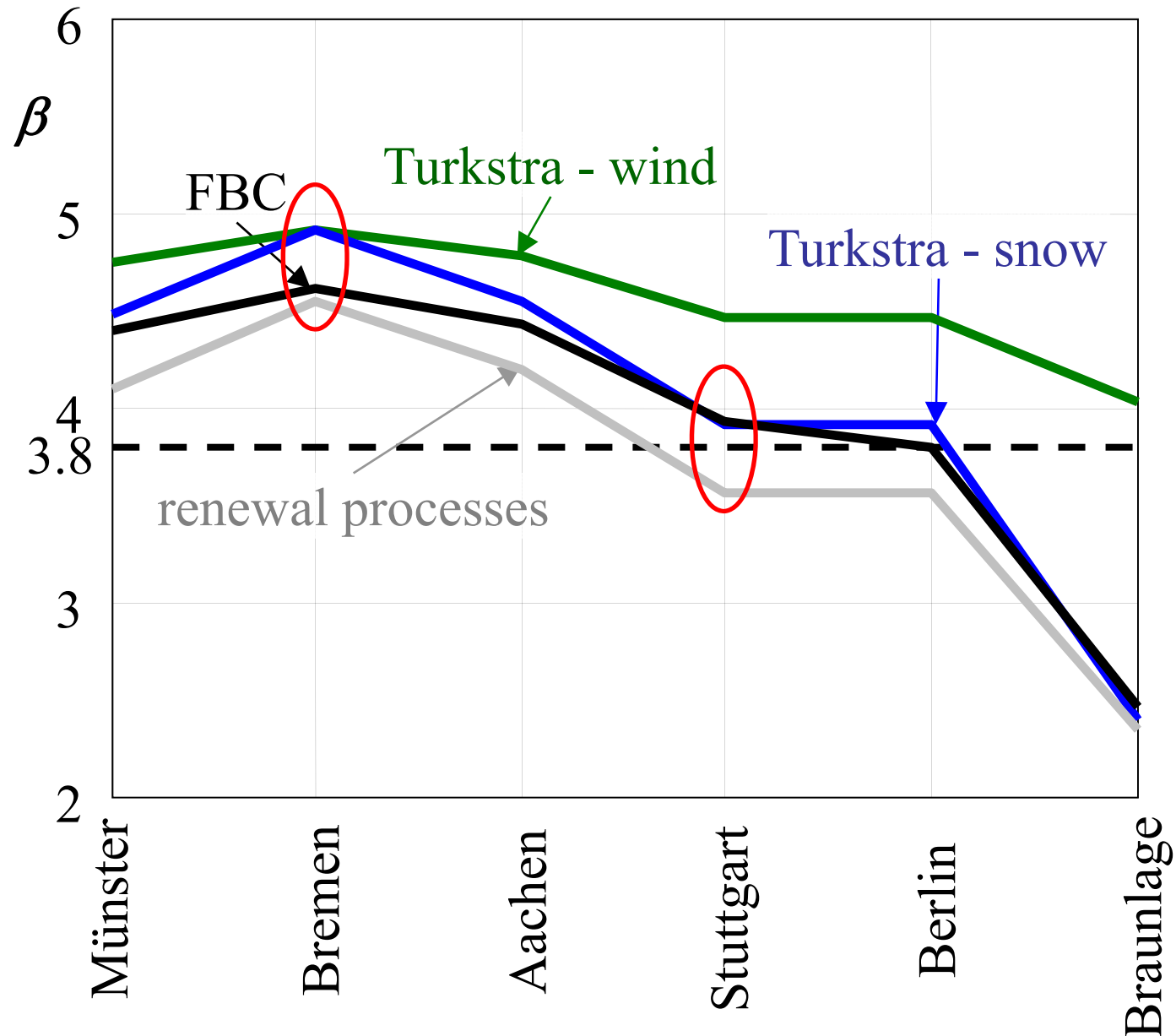
Reliability index – frame A ($\chi = 0.8$)

One dominant action (frame A – snow, frame C - wind)

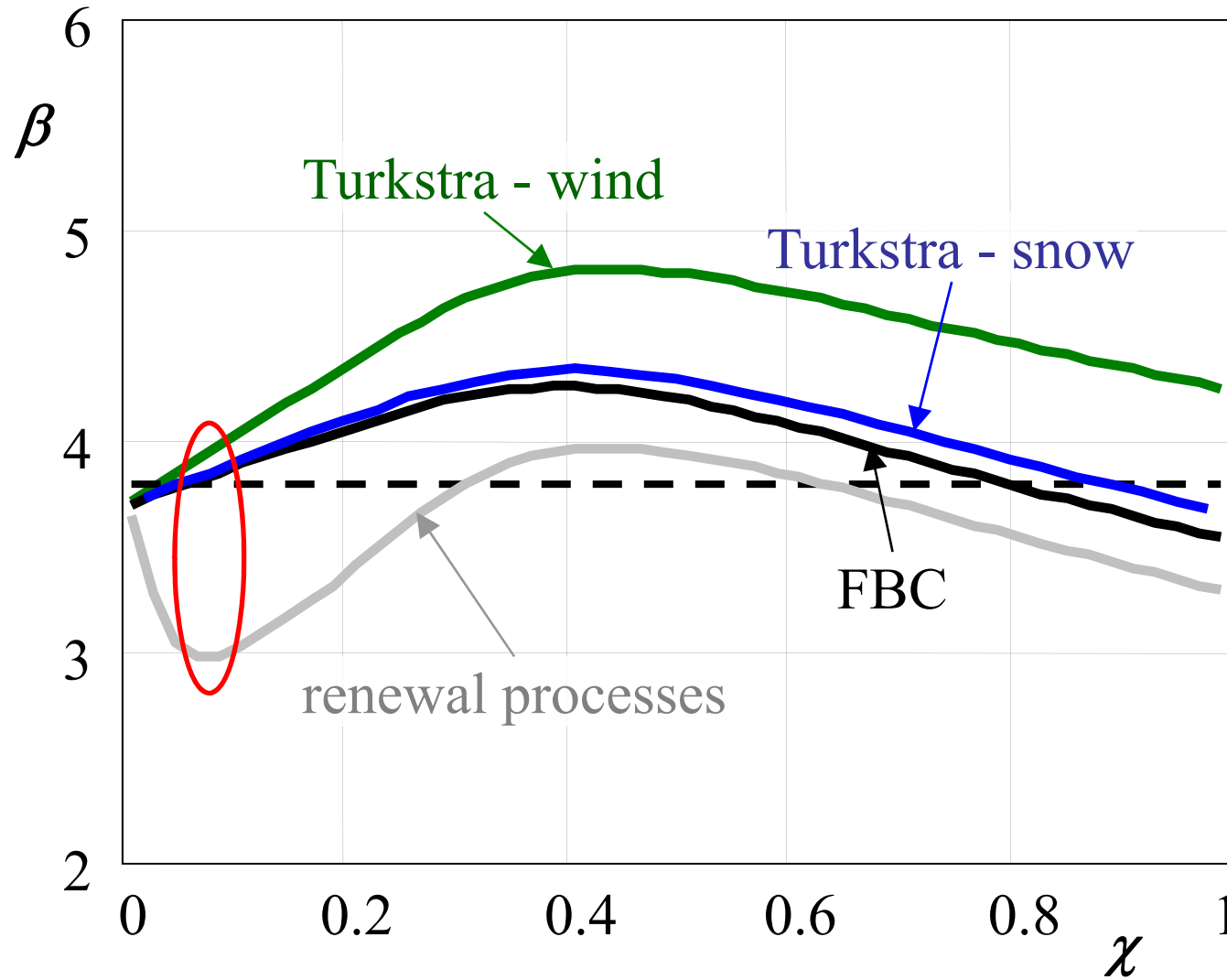


Reliability index – frame B ($\chi = 0.8$)

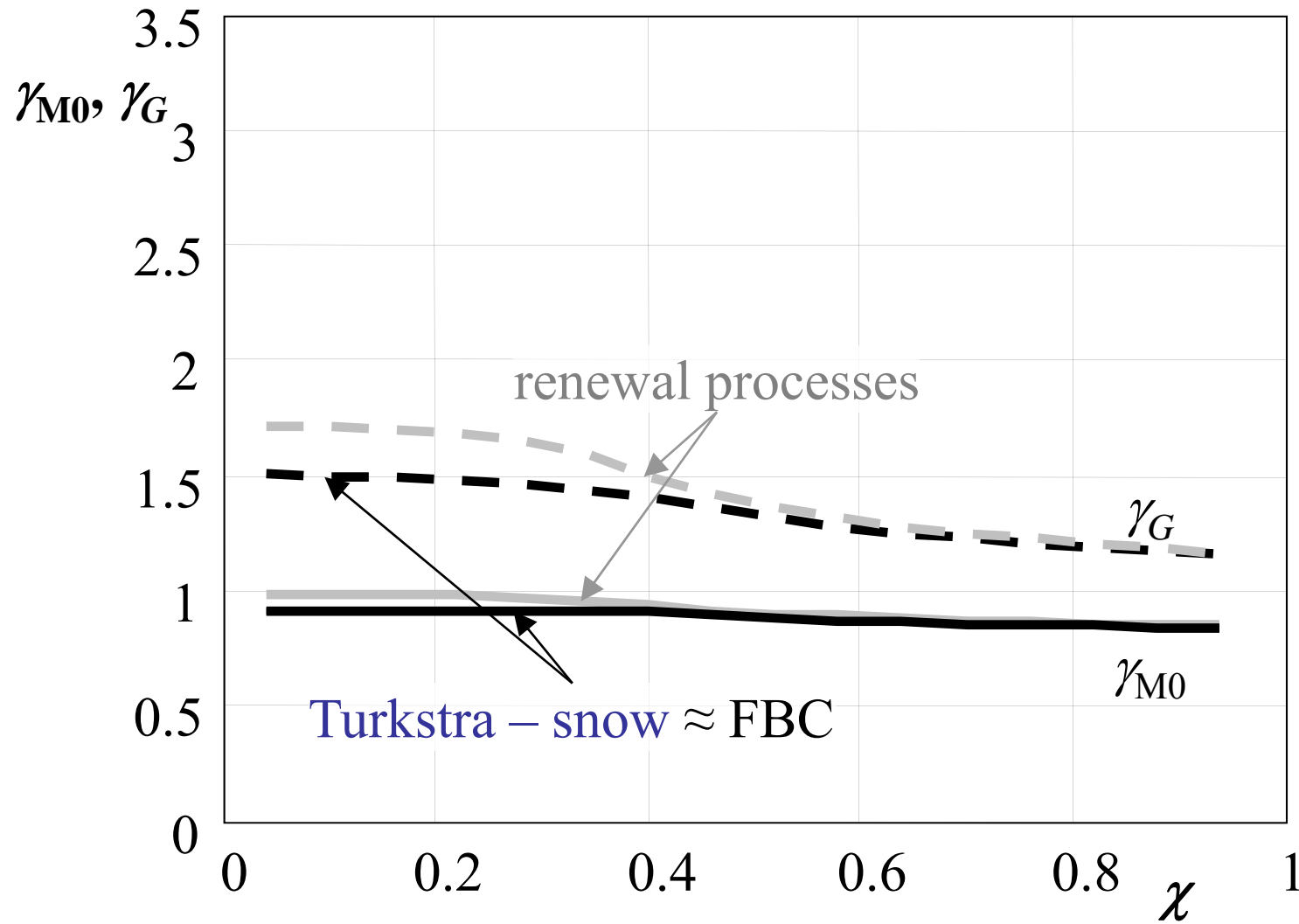
Comparable effects of snow and wind (frame B)



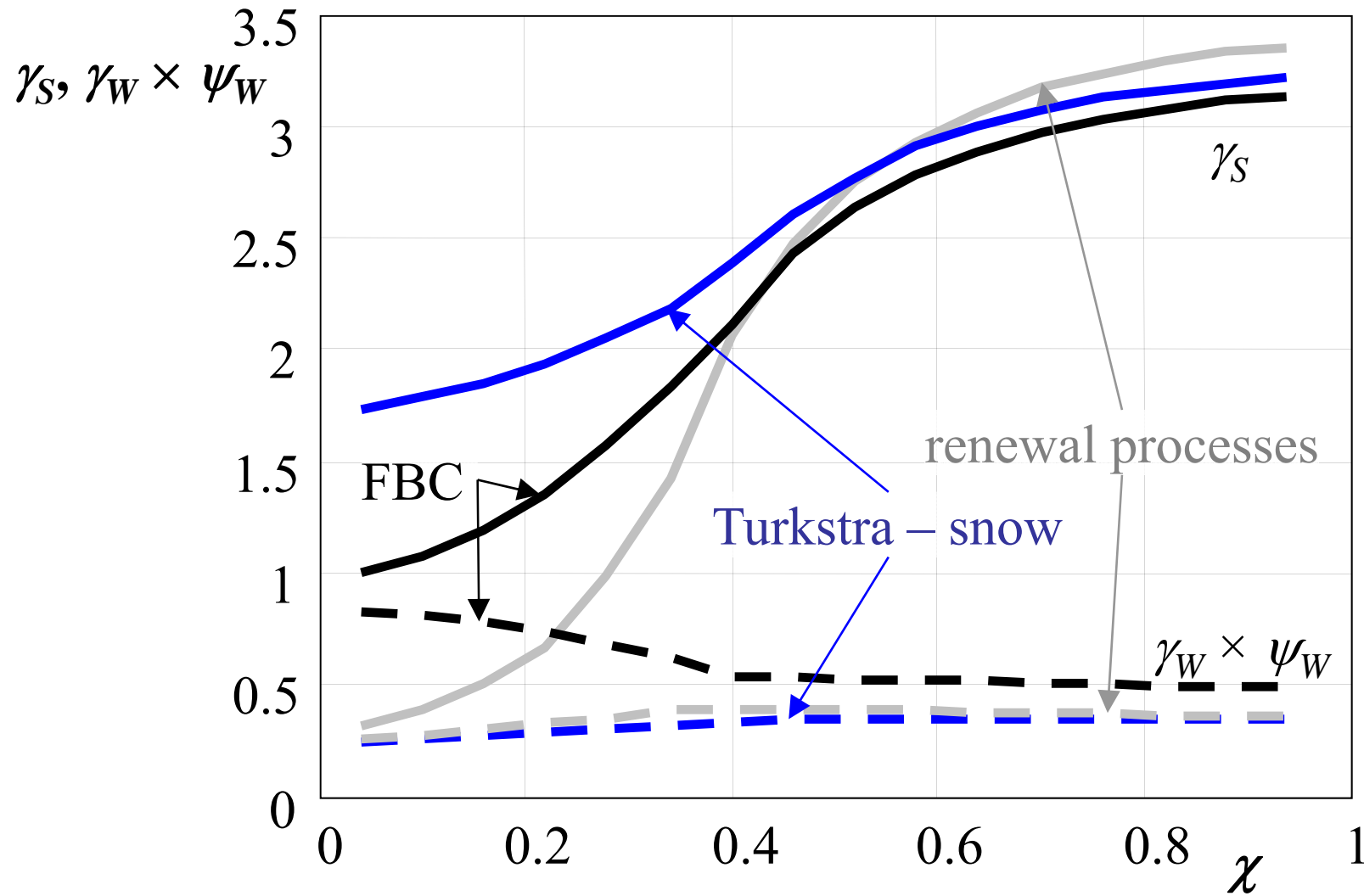
Reliability index vs. χ – frame B, Berlin



Partial factors γ_{M0} and γ_G vs. χ – frame B, Berlin ($\beta_t = 3.8$)



Partial factors γ_S and $\gamma_W \times \psi_W$ vs. χ – frame B, Berlin ($\beta_t = 3.8$)



Conclusions

- Selection of a model for the *load combination* may be a *key issue* of reliability analysis.
- *Comparison* of the three approaches reveals that:
 1. *Turkstra's rule*:
 - (+) Reliability can be assessed by *any method* for the time-invariant analysis.
 - (+) Estimation of *partial factors* is *straightforward*.
 - (0) When applied strictly as proposed, *failure probability* may be *underestimated* (error insignificant).
 2. *Ferry Borges-Castanheta models*:
 - (+) The *exact solution* is found if time-variant loads are well described by FBC models.
 - (-) *Rackwitz-Fiessler algorithm* may be *unavailable* in software.
 - (-) Estimation of *partial factors* may be *complicated*.

Conclusions

3. *Renewal processes:*

(0) Estimation of *partial factors* is *straightforward* if a dominant load case is identified.

(-) For dominant time-invariant variables, *conservative results* are obtained.

(-) *Upper bound* on failure probability is *not available* in software products.

- **For common studies, Turkstra's rule is recommended (verification by FBC models).**
- **Renewal processes may be useful for non-stationary conditions.**

More details: Sýkora, M. - Holický, M. Comparison of load combination models for probabilistic calibrations (to be published). In *Proc. ICASP11, 1-4 August, 2011, ETH Zurich, Switzerland, 2011.*



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Thank you for your attention.

References:

- Ferry Borges & Castanheta (1971) 'Structural Safety', Course 101 (2nd ed.). Lisbon: Laboratorio National de Engenharia Civil.
- Rackwitz (1998) 'Computational Techniques in Stationary and Non-Stationary Load Combination – A Review ...', J Structur Eng.
- Sykora (2005) 'Load Combination Model Based on Intermittent Rectangular Wave Renewal Processes', ICOSAR 2005.
- Turkstra (1970) 'Theory of Structural Design Decisions', University of Waterloo, Ontario, Canada