

# FINITE ELEMENT FORMULATION FOR ONE-DIMENSIONAL PROBLEMS

## AXIALLY LOADED ELASTIC BAR

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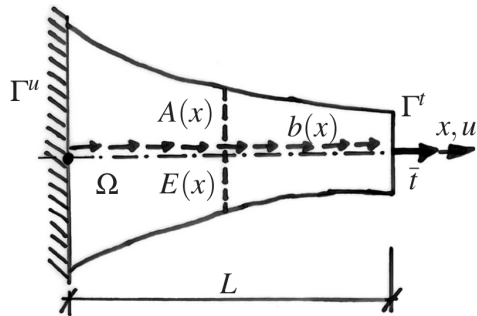


**132NAST - Numerical analysis of structures**  
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- Strong form

$$\frac{d}{dx} \left( E(x)A(x) \frac{du(x)}{dx} \right) + b(x) = 0$$

- Boundary conditions:

$$\begin{aligned} \bar{u}(x) &= u(x), & x \in \Gamma^u \\ E(x)A(x) \frac{du}{dx} n(x) &= \bar{t}(x), & x \in \Gamma^t \end{aligned}$$



- Weighted residual method:

$$\int_{\Omega} \delta u(x) \left( \frac{d}{dx} \left( E(x)A(x) \frac{du(x)}{dx} \right) + b(x) \right) dx = 0$$

- Integration by parts:

$$\int_{\Gamma} \delta u(x) E(x)A(x) \frac{du(x)}{dx} n(x) dx - \int_{\Omega} \frac{d\delta u(x)}{dx} E(x)A(x) \frac{du(x)}{dx} dx + \int_{\Omega} \delta u(x) b(x) dx = 0$$

- Integral on the boundary:

$$\int_{\Gamma_u} \underbrace{\delta u(x)}_{=0} E(x)A(x) \frac{du(x)}{dx} n(x) dx + \int_{\Gamma_t} \delta u(x) \underbrace{E(x)A(x) \frac{du(x)}{dx} n(x)}_{=\bar{t}} dx$$

- Weak form - we are looking for such an admissible trial solution  $u$  ( $u = \bar{u}$  in  $\Gamma_u$ ) to be valid:

$$\int_{\Omega} \frac{d\delta u(x)}{dx} E(x)A(x) \frac{du(x)}{dx} dx = \int_{\Omega} \delta u(x) b(x) dx + \int_{\Gamma_t} \delta u(x) \bar{t} dx, \quad \delta u = 0 \in \Gamma_u$$



- The domain is discretized by finite elements and nodes
- The weak form is expressed by an approximation of the trial solution and the weight function in elements. The approximation solution requires  $C^0$  continuity
- The displacement function  $u$  is approximated in each element in the following shape:

$$u^e = \mathbf{N}^e \mathbf{d}^e, \quad u^e = \bar{u} \quad \text{in} \quad \Gamma_u$$

where  $\mathbf{d}^e$  is the vector of nodal displacements.

- The weight function  $u$  is approximated in each element in the same way:

$$\delta u^e = \mathbf{N}^e \mathbf{w}^e, \quad \delta u^e = 0 \quad \text{in} \quad \Gamma_u$$

where  $\mathbf{w}^e$  is the vector of nodal weight function.

- The integral of the weak form is transferred into the sum of integrals in elements:

$$\sum_{e=1}^n \left\{ \int_{x_1^e}^{x_2^e} \frac{d\delta u^e}{dx} E(x) A(x) \frac{du^e}{dx} dx - \int_{x_1^e}^{x_2^e} \delta u^e b(x) dx - (\delta u^e \bar{t})|_{\Gamma_t} \right\} = 0$$



- Derivatives of approximated functions:

$$\begin{aligned}\frac{du^e}{dx} &= \frac{dN^e}{dx} d^e = B^e d^e \\ \frac{d\delta u^e}{dx} &= \frac{dN^e}{dx} w^e = B^e w^e\end{aligned}$$

- Introducing approximations into the previous weak form gives:

$$\sum_{e=1}^n w^{eT} \left\{ \underbrace{\int_{x_1^e}^{x_2^e} B^{eT} E A B^e dx}_{K^e} d^e - \underbrace{\int_{x_1^e}^{x_2^e} N^{eT} b dx}_{f_{\Omega}} - \underbrace{\left( N^{eT} \bar{t} \right) |_{\Gamma_t}}_{f_{\Gamma_t}} \right\} = 0$$



- If local vectors  $\mathbf{d}^e$ ,  $\mathbf{w}^e$  are expanded into global vectors of nodal values  $\mathbf{d}$ ,  $\mathbf{w}$ , we can write:

$$\mathbf{w}^T \left( \sum_{e=1}^n \tilde{\mathbf{K}}^e \mathbf{d} - \sum_{e=1}^n \tilde{\mathbf{f}}^e \right) = 0, \quad \forall \mathbf{w}, \quad w = 0 \in \Gamma_u$$

$$\mathbf{w}^T (\mathbf{K} \mathbf{d} - \mathbf{f}) = 0$$

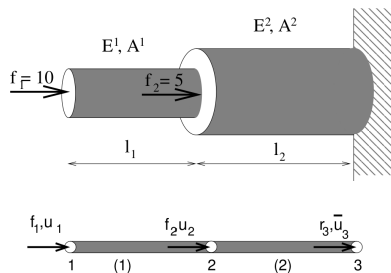
- Finally, we have:

$$\mathbf{K} \mathbf{d} - \mathbf{f} = 0$$

- Note: Residuum:

$$\mathbf{R} = \mathbf{K} \mathbf{d} - \mathbf{f}$$





$$\mathbf{w}^T \mathbf{R} = 0, \quad \forall \mathbf{w}, \quad \text{except } w = 0 \in \Gamma_u$$

$$\begin{aligned} w_1 R_1 + w_2 R_2 &= 0, & w_1, w_2 &\implies R_1 = R_2 = 0 \\ R_3 &\neq 0, & w_3 &= 0 \end{aligned}$$

$$\mathbf{R} = \begin{Bmatrix} 0 \\ 0 \\ R_3 \end{Bmatrix} = \begin{bmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ \bar{u}_3 \end{Bmatrix} - \begin{Bmatrix} f_1 \\ f_2 \\ f_3 \end{Bmatrix}$$

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ \bar{u}_3 \end{Bmatrix} = \begin{Bmatrix} f_1 \\ f_2 \\ f_3 + R_3 \end{Bmatrix}$$





Stiffness matrix of an element with linear approximation functions:

- Matrix of interpolation functions:

$$\mathbf{N}^e = \frac{1}{l^e} [x_2^e - x, x - x_1^e]$$

- Geometric matrix:

$$\mathbf{B}^e = \frac{d\mathbf{N}^e}{dx} = \frac{1}{l^e} [-1, 1]$$

- Introducing the geometric matrix into the element stiffness matrix gives:

$$\begin{aligned} \mathbf{K}^e &= \int_{x_1^e}^{x_2^e} \mathbf{B}^{eT} E A \mathbf{B}^e dx = \int_{x_1^e}^{x_2^e} \frac{1}{l^e} \begin{bmatrix} -1 \\ 1 \end{bmatrix} E A \frac{1}{l^e} [-1, 1] dx \\ \mathbf{K}^e &= \frac{EA}{l^e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \end{aligned}$$



Loading vector in an element with linear approximation functions:

- Loading vector:

$$\mathbf{f}_{\Omega}^e = \int_{x_1^e}^{x_2^e} \mathbf{N}^{eT} b(x) dx$$

- In the case of linear volume loading  $b(x)$ , the loading can be expressed by approximation functions:

$$b(x) = \mathbf{N}^e \mathbf{b}^e$$

- The introduction of the previous equation into the loading vector integral leads to the following relationship:

$$\begin{aligned} \mathbf{f}_{\Omega}^e &= \int_{x_1^e}^{x_2^e} \mathbf{N}^{eT} \mathbf{N}^e dx \mathbf{b}^e \\ &= \frac{1}{l^e} \int_{x_1^e}^{x_2^e} \begin{bmatrix} (x_2^e - x)^2 & (x_2^e - x)(x - x_1^e) \\ (x_2^e - x)(x - x_1^e) & (x - x_1^e)^2 \end{bmatrix} dx \mathbf{b}^e \\ &= \frac{l^e}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{Bmatrix} b_1^e \\ b_2^e \end{Bmatrix} \end{aligned}$$



## Element with Linear Approximation in Natural Coordinates:

- The matrix of interpolation functions in the natural coordinate system is given by this expression:

$$\mathbf{N}^e = \left[ \frac{1}{2}(1 - \xi), \frac{1}{2}(1 + \xi) \right]$$

- Derivatives of shape functions with respect to  $x$  are needed for the geometric matrix  $\mathbf{B}^e$  calculation. The derivative of a compound function reads:

$$\frac{df}{d\xi} = \frac{df}{dx} \frac{dx}{d\xi}$$

- The inverse relation is:

$$\frac{df}{dx} = \left( \frac{dx}{d\xi} \right)^{-1} \frac{df}{d\xi}$$



- The dependence of  $x$  on  $\xi$  can be taken from the isoparametric element definition:

$$x(\xi) = \mathbf{N}^e(\xi)\mathbf{x}^e, \quad dx(\xi) = \frac{d\mathbf{N}^e}{d\xi}\mathbf{x}^e d\xi = Jd\xi$$

- In the case of the linear approximation:

$$\begin{aligned} x(\xi) &= \frac{1}{2}(1 - \xi) \cdot x_1^e + \frac{1}{2}(1 + \xi) \cdot x_2^e \\ dx(\xi) &= \frac{1}{2}(x_2^e - x_1^e)d\xi = \frac{l^e}{2}d\xi \end{aligned}$$

- Introducing the previous equations into the element stiffness matrix:

$$\begin{aligned} \mathbf{K}^e &= \int_{-1}^1 \mathbf{B}^{eT} E A \mathbf{B}^e J d\xi = \int_{-1}^1 \begin{bmatrix} -1/l^e \\ 1/l^e \end{bmatrix} E A \begin{bmatrix} -1 & 1 \\ l^e & l^e \end{bmatrix} \frac{l^e}{2} d\xi \\ \mathbf{K}^e &= \frac{EA}{l^e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \end{aligned}$$



Element with Quadratic Approximation in Natural Coordinates:

- Matrix of interpolation functions in natural coordinate system:

$$\mathbf{N}^e = \left[ -\frac{1}{2}(1 - \xi) + \frac{1}{2}(1 - \xi)^2, (1 - \xi^2), \frac{1}{2}(1 + \xi) - \frac{1}{2}(1 - \xi^2) \right] \quad (1)$$

- Approximation of coordinates:

$$x(\xi) = \mathbf{N}^e(\xi)\mathbf{x}^e = \left( -\frac{1}{2}\xi + \frac{1}{2}\xi^2 \right) \cdot x_1^e + (1 - \xi^2) \cdot x_2^e + \left( \frac{1}{2}\xi + \frac{1}{2}\xi^2 \right) \cdot x_3^e$$

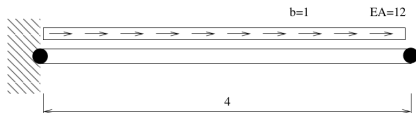
$$J = \frac{dx}{d\xi} = \left( -\frac{1}{2} + \xi \right) \cdot x_1^e - 2\xi \cdot x_2^e + \left( \frac{1}{2} + \xi \right) \cdot x_3^e$$

$$\mathbf{K}^e = \int_{-1}^1 \mathbf{B}^{eT} \mathbf{E} \mathbf{A} \mathbf{B}^e J d\xi$$

$$\mathbf{f}^e = \int_{-1}^1 \mathbf{N}^{eT} b(\xi) J d\xi$$



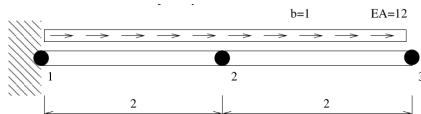
Example:



- Analytical solution - Strong form:
- Integration:
- Integration constant from boundary conditions:
- Solution:



Example:



- Solution with one quadratic element:

$$\mathbf{x}^e = \{0; 2; 4\}^T$$

$$\mathbf{N}^e = \left[ \frac{1}{2}(1 - \xi) - \frac{1}{2}(1 - \xi)^2, (1 - \xi^2), \frac{1}{2}(1 + \xi) - \frac{1}{2}(1 - \xi^2) \right]$$

$$x = \mathbf{N}^e \mathbf{x}^e = 2 + 2 \cdot \xi \implies \frac{dx}{d\xi} = 2, \quad J = 2$$

$$\frac{d\mathbf{N}^e}{d\xi} = \left[ -\frac{1}{2} + \xi, -2\xi, \frac{1}{2} + \xi \right]$$

$$\mathbf{B}^e = \frac{d\mathbf{N}^e}{d\xi} \frac{d\xi}{dx} = \left[ -\frac{1}{4} + \frac{1}{2}\xi, -\xi, \frac{1}{4} + \frac{1}{2}\xi \right]$$



$$\mathbf{K}^e = \int_{-1}^1 \mathbf{B}^{eT} E A \mathbf{B}^e J d\xi = \frac{EA}{12} \begin{bmatrix} 7 & -8 & 1 \\ -8 & 16 & -8 \\ 1 & -8 & 7 \end{bmatrix}$$

$$\mathbf{f}^e = \int_{-1}^1 \mathbf{N}^{eT} b J d\xi = \{2/3; 8/3; 2/3\}^T$$

$$\mathbf{f} = \{8/3; 2/3\}^T$$

$$\mathbf{K} = \begin{bmatrix} 16 & -8 \\ -8 & 7 \end{bmatrix}$$

$$\mathbf{d} = \mathbf{K}^{-1} \mathbf{f} = \{1/2; 2/3\}^T$$

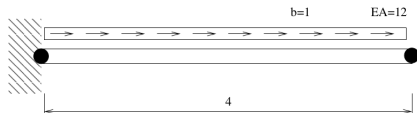
$$\varepsilon = \frac{du}{dx} = \mathbf{B}^e \mathbf{d}^e = \mathbf{B} \{0, 1/2; 2/3\}^T = 1/6 - 1/6 \cdot \xi$$

$$R_1 = \mathbf{K}^e(1) \mathbf{d}^e - \mathbf{f}^e(1) = \{7; -8; 1\} \{0; 1/2; 2/3\}^T - 2/3 = -4 \quad (2)$$





Example:



- Solution with one linear element:

$$\mathbf{K}^e = \frac{EA}{l} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 3 & -3 \\ -3 & 3 \end{bmatrix}$$

$$\mathbf{f}^e = \int_0^4 \mathbf{N}^{eT} b dx = \{2; 2\}^T$$

(3)

- System of equations:

$$\mathbf{d} = \mathbf{K}^{-1} \mathbf{f} \implies \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} = \begin{bmatrix} 3 & -3 \\ -3 & 3 \end{bmatrix}^{-1} \begin{Bmatrix} 2 \\ 2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 2/3 \end{Bmatrix}$$

(4)



- English course of “Numerical analysis of structures” by J. Zeman (jan.zeman@fsv.cvut.cz)
- Czech course of “Numerická analýza konstrukcí” (Numerical analysis of structures) by B. Patzák (borek.patzak@fsv.cvut.cz)
- J. Fish and T. Belytschko: A First Course in Finite Elements, John Wiley & Sons, 2007

