

FINITE ELEMENT FORMULATION FOR 2D PROBLEMS - LINEAR ELASTICITY

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1 BASIC EQUATIONS

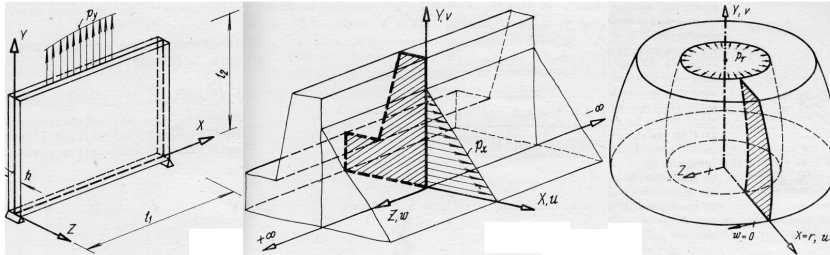
- Kinematic equations
- Equilibrium equations
- Constitutive equations
- Boundary conditions

2 WEAK FORM

3 FEM DISCRETIZATION



- Basic assumptions for linear elasticity:
 - Deformations are small
 - The behavior of the material is linear
 - Dynamic effects are neglected
 - No gaps or overlaps occur during the deformation of the solid



- Quantities (geometry, material properties, loading) are independent of one coordinate (dimension):
 - Plane stress (historically, the first FEM application [Turner, 1956])
 - Plane strain
 - Axisymmetric problem



- Coordinate vector:

$$\mathbf{x} = \{x, y\}^T$$

- Displacements:

$$\mathbf{u}(\mathbf{x}) = \{u(\mathbf{x}), v(\mathbf{x})\}^T$$

- Strain vector (independent components):

$$\boldsymbol{\varepsilon}(\mathbf{x}) = \{\varepsilon_x(\mathbf{x}), \varepsilon_y(\mathbf{x}), \gamma_{xy}(\mathbf{x})\}^T$$

- Kinematic equations:

$$\begin{Bmatrix} \varepsilon_x(\mathbf{x}) \\ \varepsilon_y(\mathbf{x}) \\ \gamma_{xy}(\mathbf{x}) \end{Bmatrix} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial x} & \frac{\partial}{\partial y} \end{bmatrix} \begin{Bmatrix} u(\mathbf{x}) \\ v(\mathbf{x}) \end{Bmatrix},$$

$$\boldsymbol{\varepsilon}(\mathbf{x}) = \boldsymbol{\partial}^T \mathbf{u}(\mathbf{x})$$

- In planestrain, $\varepsilon_z = 0$, and in planestress, $\varepsilon_z \neq 0$ is calculated from constitutive equations



- Stress vector (independent components):

$$\boldsymbol{\sigma}(\mathbf{x}) = \{\sigma_x(\mathbf{x}), \sigma_y(\mathbf{x}), \tau_{xy}(\mathbf{x})\}^T$$

- Equilibrium equation (static equations):

$$\begin{bmatrix} \frac{\partial}{\partial x} & 0 & \frac{\partial}{\partial y} \\ 0 & \frac{\partial}{\partial x} & \frac{\partial}{\partial y} \end{bmatrix} \begin{Bmatrix} \sigma_x(\mathbf{x}) \\ \sigma_y(\mathbf{x}) \\ \tau_{xy}(\mathbf{x}) \end{Bmatrix} + \begin{Bmatrix} \bar{X}(\mathbf{x}) \\ \bar{Y}(\mathbf{x}) \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

$$\boldsymbol{\partial}\boldsymbol{\sigma}(\mathbf{x}) + \bar{\mathbf{X}}(\mathbf{x}) = \mathbf{0}$$

- In planestress, $\sigma_z = 0$, and in planestrain, $\sigma_z \neq 0$ is calculated from constitutive equations



■ Plane stress:

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

$$\varepsilon_z = -\frac{\nu}{E}(\varepsilon_x + \varepsilon_y)$$

■ Plane strain:

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix}$$

$$\sigma_z = \frac{E}{1+\nu} \left[\frac{\nu}{1-2\nu}(\varepsilon_x + \varepsilon_y) \right]$$



- Kinematic boundary conditions:

$$\mathbf{u}(\mathbf{x}) - \bar{\mathbf{u}}(\mathbf{x}) = \mathbf{0}, \quad \mathbf{x} \in \Gamma_u$$

- Static boundary conditions (prescribed tractions) $\mathbf{x} \in \Gamma_p$:

$$\begin{bmatrix} n_x(\mathbf{x}) & 0 & n_y(\mathbf{x}) \\ 0 & n_y(\mathbf{x}) & n_x(\mathbf{x}) \end{bmatrix} \begin{Bmatrix} \sigma_x(\mathbf{x}) \\ \sigma_y(\mathbf{x}) \\ \tau_{xy}(\mathbf{x}) \end{Bmatrix} - \begin{Bmatrix} \bar{p}_x(\mathbf{x}) \\ \bar{p}_y(\mathbf{x}) \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

$$\mathbf{n}(\mathbf{x})\boldsymbol{\sigma}(\mathbf{x}) - \bar{\mathbf{p}}(\mathbf{x}) = \mathbf{0}$$

- The divergence (Clapeyron) theorem:

$$\int_{\Omega} \boldsymbol{\sigma}^T \boldsymbol{\partial}^T \mathbf{u} d\Omega = \int_{\Gamma} \mathbf{u}^T \mathbf{n} \boldsymbol{\sigma} d\Gamma - \int_{\Omega} \mathbf{u}^T \boldsymbol{\partial} \boldsymbol{\sigma} d\Omega$$



Weighted residual method (Galerkin method):

- For an arbitrary weight function $\delta \mathbf{u}$ so that $\delta \mathbf{u}(\mathbf{x}) = \mathbf{0}$ for $\mathbf{x} \in \Gamma_u$, it holds:

$$\int_{\Omega} \delta \mathbf{u}(\mathbf{x})^T (\partial \boldsymbol{\sigma}(\mathbf{x}) + \bar{\mathbf{X}}) d\Omega = 0$$

Clapeyron theorem:

$$\int_{\Gamma_u} \overbrace{\delta \mathbf{u}(\mathbf{x})^T \mathbf{n}(\mathbf{x}) \boldsymbol{\sigma}(\mathbf{x})}^{=0} d\Gamma + \int_{\Gamma_p} \delta \mathbf{u}(\mathbf{x})^T \overbrace{\mathbf{n}(\mathbf{x}) \boldsymbol{\sigma}(\mathbf{x})}^{=\bar{\mathbf{p}}} d\Gamma - \int_{\Omega} (\partial^T \delta \mathbf{u}(\mathbf{x}))^T \boldsymbol{\sigma}(\mathbf{x}) d\Omega + \int_{\Omega} \delta \mathbf{u}(\mathbf{x})^T \bar{\mathbf{X}} d\Omega = 0$$

- If the weight function $\delta \mathbf{u}$ has the physical meaning of **the virtual displacement**, the expression $\partial^T \delta \mathbf{u}(\mathbf{x})$ can be identified as **the virtual strain** $\delta \boldsymbol{\varepsilon}(\mathbf{x})$

$$\begin{aligned} \int_{\Omega} \delta \boldsymbol{\varepsilon}(\mathbf{x})^T \boldsymbol{\sigma}(\mathbf{x}) d\Omega &= \int_{\Gamma_p} \delta \mathbf{u}(\mathbf{x})^T \bar{\mathbf{p}}(\mathbf{x}) d\Gamma + \int_{\Omega} \delta \mathbf{u}(\mathbf{x})^T \bar{\mathbf{X}} d\Omega, \\ \delta W_{\text{int}} &= \delta W_{\text{ext}} \end{aligned}$$

- The weighted residual method can be explained as the generalisation of **the virtual displacement principle**



Finite Element Method:

- The domain Ω is discretized by finite elements and nodes
- The weak form is expressed by an approximation of the trial solution and the weight function in elements
- Displacement approximation:

$$\mathbf{u}^e(\mathbf{x}) \approx \mathbf{N}^e(\mathbf{x})\mathbf{d}^e$$

- Approximations of strains and stresses:

$$\begin{aligned}\boldsymbol{\varepsilon}^e(\mathbf{x}) &\approx \mathbf{B}^e(\mathbf{x})\mathbf{d}^e, \\ \boldsymbol{\sigma}^e(\mathbf{x}) &\approx \mathbf{D}^e(\mathbf{x})(\mathbf{B}^e(\mathbf{x})\mathbf{d}^e)\end{aligned}$$

- Weight functions approximation:

$$\begin{aligned}\delta\mathbf{u}^e(\mathbf{x}) &\approx \mathbf{N}^e(\mathbf{x})\mathbf{w}^e, \\ \delta\boldsymbol{\varepsilon}^e(\mathbf{x}) &\approx \mathbf{B}^e(\mathbf{x})\mathbf{w}^e\end{aligned}$$



Finite Element Method:

- Introducing approximations of trial solution and weight function into the weak form (for all w^e that $w^e = 0$ on Γ_T), we obtain the following equation, where the integrals of the weak form are transferred into the sum of integrals in elements:

$$\sum_{e=1}^n w^{eT} \left\{ \overbrace{\int_{\Omega^e} B^{eT}(x) D^e(x) B^e(x) d\Omega}^{K^e} d^e - \overbrace{\int_{\Gamma^e} N^{eT}(x) N^e(x) d\Gamma}^{f_{\Gamma}^e} \bar{p}^e - \overbrace{\int_{\Omega^e} N^{eT}(x) N^e(x) d\Omega}^{f_{\Omega}^e} \bar{X}^e \right\} = 0$$



- Global quantities - localization, for which is introduced the distribution function for an each element: L^e such that it is valid $d^e = L^e d$:

$$\mathbf{w}^T \sum_{e=1}^n (L^{eT} \mathbf{K}^e L^e d - L^{eT} \mathbf{f}_\Gamma^e - L^{eT} \mathbf{f}_\Omega^e) = 0,$$

We can write:

$$\mathbf{w}^T \left(\sum_{e=1}^n \hat{\mathbf{K}}^e d - \sum_{e=1}^n \hat{\mathbf{f}}^e \right) = 0$$

- Finally, we have:

$$\mathbf{K}d = \mathbf{f}$$



- Decomposition of the stiffness matrix according to constrained and prescribed DOF's (\mathbf{d}_u):

$$\begin{bmatrix} \mathbf{K}_{pp} & \mathbf{K}_{pu} \\ \mathbf{K}_{up} & \mathbf{K}_{uu} \end{bmatrix} \begin{bmatrix} \mathbf{d}_p \\ \mathbf{d}_u \end{bmatrix} = \begin{bmatrix} \mathbf{f}_p \\ \mathbf{f}_u \end{bmatrix} + \begin{bmatrix} \mathbf{0} \\ \mathbf{R} \end{bmatrix},$$

where

$$\mathbf{K}_{pp}\mathbf{d}_p = \mathbf{f}_p - \mathbf{K}_{pu}\mathbf{d}_u$$

and residuum vector - vector of nodal heat fluxes on the boundary:

$$\mathbf{R} = \mathbf{K}_{up}\mathbf{d}_p + \mathbf{K}_{uu}\mathbf{d}_u - \mathbf{f}_u$$



- English course of “Numerical analysis of structures” by J. Zeman (jan.zeman@fsv.cvut.cz)
- Czech course of “Numerická analýza konstrukcí” (Numerical analysis of structures) by B. Patzák (borek.patzak@fsv.cvut.cz)
- J. Fish and T. Belytschko: A First Course in Finite Elements, John Wiley & Sons, 2007

